

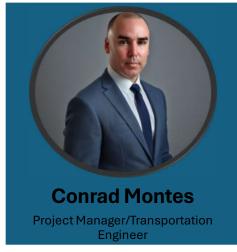


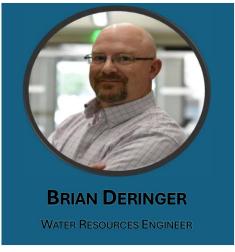
Leon Valley Baptist Church Recreation Center



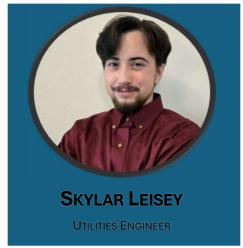
UTSA Engineering

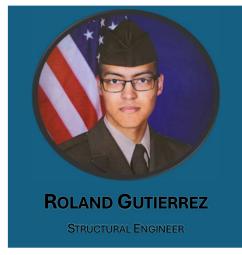
Team members













UTSA Engineering

Project Overview

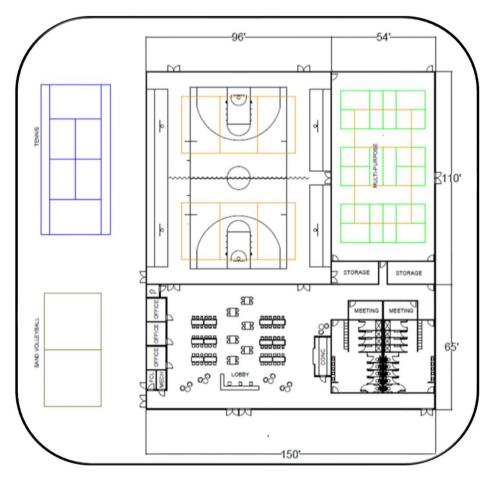


Indoor Amenities:

- Indoor Basketball Court
- Pickleball Court
- Multipurpose Rooms
- Bathrooms/Showers
- Lobby & Storage Areas
- ADA Accessibility

Outdoor Amenities:

- Tennis Court
- Sand Volleyball Pit
- Ample Parking

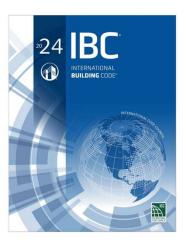


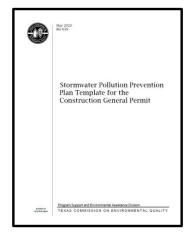
Project Location

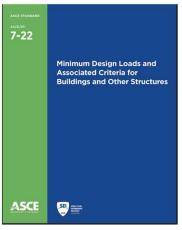


Applicable Codes

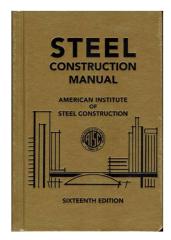
- COSA Unified Development Code (UDC)
- 2024 International Building Code (IBC)
- ASCE 7-22
- AISC 360-16
- ACI 318-18
- TMS 402
- COSA Storm Water Design Criteria Manual, June 2025
- 2018 International Fire Code
- Texas Accessibility Standards (TAS)
- TCEQ
- FWS
- TPWD





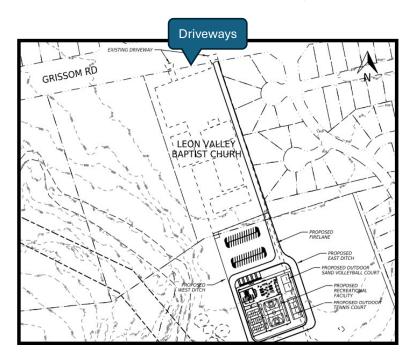


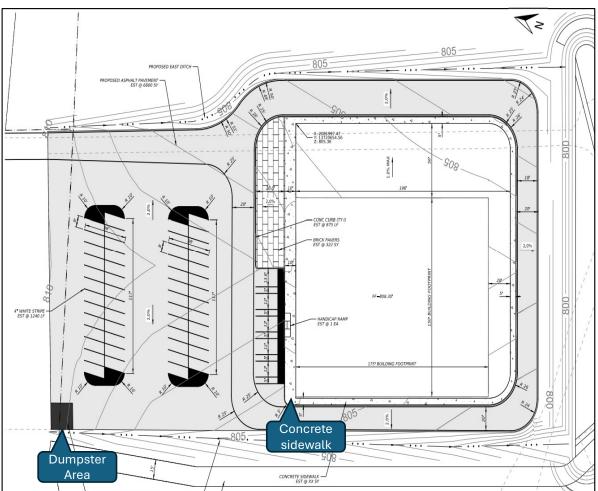




Site Plan

- 2 24' Existing Full Access Driveways
- Building (150' x 175') ~26,250 SF
- Hot Mix Asphalt Pavement
- Concrete sidewalk around bldg.





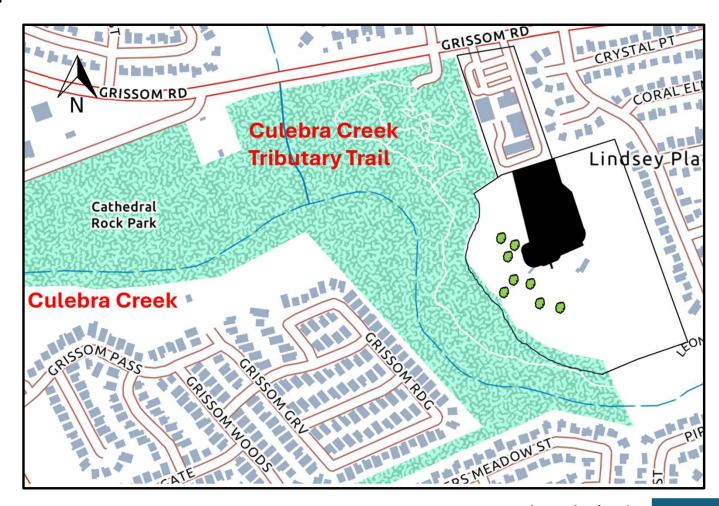
Land Development

- Zoned R-6
- Surrounded by residential & Mixed-residential
- UDC SEC-35-311
- Major Plat/ Church owned property



Environmental

- NPS / National Environmental Protection Act 1970
- Clean Water Act 1972 Section 404
- 1 acre +< TCEQ / COSA-General Permit to Discharge under the Texas Pollutant Discharge Elimination System
- SW3P



Tree Protection Plan

- TREE REMOVAL PERMIT APPLICATION/STAND DELINEATION
- NON- FLOODPLAIN AREA REQ.= 38% PROVIDED = 82.5%
- FLOOD PLAIN AREA REQ. = 80%
 PROVIDED = 100%
- HERITAGE REQ. = 100%
 PROVIDED = 100% (8 TREES)
- NO MITIGATION TREES NECESSARY
- ROOT PROTECTION ZONE: 4' OFFSET ORANGE FENCING 34' PROVIDED

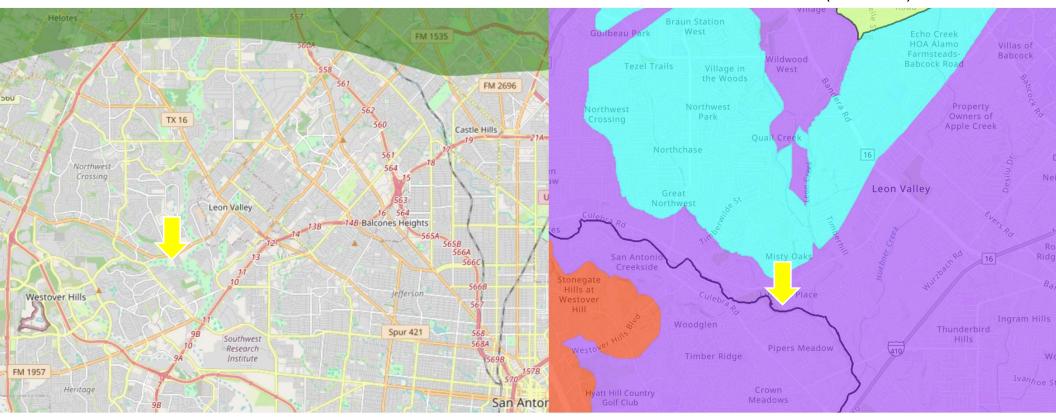




Habitat Compliance Form

NOT WITHIN CRITICAL OR LIKELY HABITAT OF GOLDEN CHEEKED WARBLER PER TPWD

NOT WITHIN KARST ZONES 1OR 2 PER FWS (ZONE 3B)



UTSA Engineering

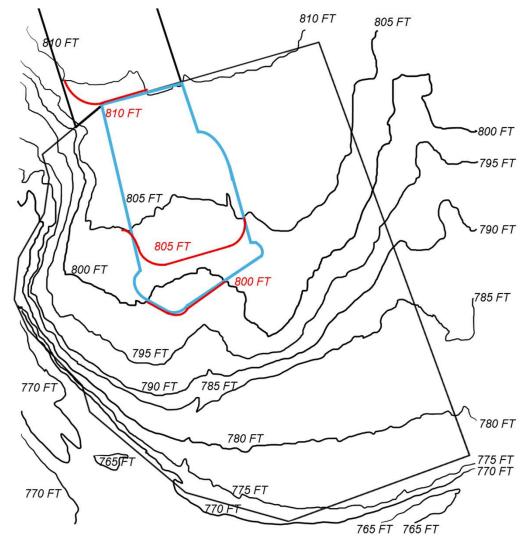
Grading

EXCAVATION: 4453 CY

• FILL: 7891 CY

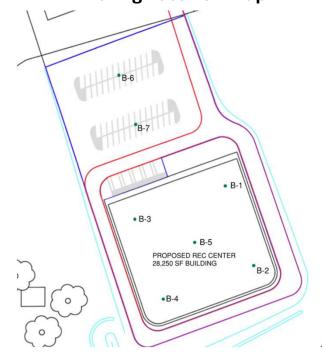
2% PAVEMENT SLOPES

• FLOOR ELEV. = 808.3 FT



Geotechnical

Boring Location Map



0-3 FAT CLAY-Stiff to very stiff Plasticity Index:

Boring 1-7 34 to 67

3-6 WHITE CHALK- Stiff to very stiff Plasticity Index:

Boring 1-7 25 to 42

6-15 LEAN CLAY – Very stiff to hard Plasticity Index:

Boring 1-7 25 to 42

Table 3

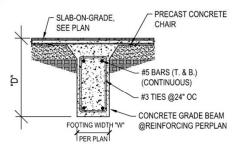
Soil Type	Moisture Content Range (%)	ontent Range Percent Passing Liquid Limit		Plastic Limit (%)	Plasticity Index	
Sandy Clay/Clayey Sand	5 to 19		48 to 55	20 to 24	27 to 31	
Fat Clay	9 to 11		55 to 97	21 to 33	34 to 67	
Lean Clay/Chalk	3 to 18	98	45 to 60	16 to 23	25 to 42	

UTSA Engineering

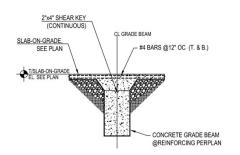
5	Styron Reyes Geotechnical, LLC					200						
2	16100 Henderson Pass #160 San Antonio, Texas 78232 Telephone: 210.857.9377					SUF	KINC	N۱ و	UMI	PAG	ξ B-	03
NT_		PROJEC	T NAME									
ECT	NUMBER	PROJECT LOCATION										
STAR	RTED 8/13/25 COMPLETED 8/13/25	GROUN	DELEV	ATION		_	HOLE	SIZE	5*			
		GROUN	D WATE	R LEV	ÆLS:							
												_
												_
S _G:	oundwater not encountered during drilling operations.	AF	TER DRI	LLING		_	_					_
o			N R	% %	တယ်	Ä,	WT.	# (£	AI		3	CONTENT
GRAPHI	MATERIAL DESCRIPTION		SAMPLE T	RECOVER (RQD)	BLOW COUNT (N VALU	POCKET P	DRY UNIT (pcf)	MOISTUR	LIQUID	PLASTIC	INDEX	FINES CON
///	Stratum I - Stiff to very stiff, dark grayish brown FAT CLAY (C	CH)	1		8-6-8							-
			N 7		(14)			19	97	30	67	
	Steature II. Many etill to have until CNALK		V ss		10-10-12							
÷	GORGEN II - VOLY SEIT IO FIREU, WESTE COVER		N 2		(22)							
芸			V ss		11-14-22			13	49	23	26	
P			M J		(50)							
	- with calcareous deposits from 6 to 10 feet		SS 4		11-22-24 (46)							
	- becomes tan and light gray in color below 8 feet											
			SS 5		8-13-16 (29)			18				
	- becomes marly below 12 feet											
	•											
			SS 6		17-37- 50/3*			12	49	19	30	
	Bottom of hole at 15.0 feet.											
֡	STAF LING (LING I SED B	LING CONTRACTOR BEA LING METHOD Dry Auger SED BY D. Rocha CHECKED BY R. Burge S Groundwater not encountered during drilling operations. MATERIAL DESCRIPTION Stratum II - Stiff to very stiff, dark grayish brown FAT CLAY (II Stratum III - Very stiff to hard, white CHALK Stratum III - Very stiff to hard, tan LEAN CLAY (CL) - with calcareous deposits from 6 to 10 feet - becomes tan and light gray in color below 8 feet - becomes marry below 12 feet	STARTED 8/13/25 COMPLETED 8/13/25 GROUN LING CONTRACTOR BEA LING METHOD _Drv Auger LED BY _D. Rocha CHECKED BY _R. Burge AT LED BY _D. Rocha CHECKED BY _R. Burg	STRATED 8/13/25 COMPLETED 8/13/25 GROUND ELEVI LING CONTRACTOR BEA LING METHOD Dry Auger SED BY D. Rocha CHECKED BY R. Burge SED Groundwater not encountered during drilling operations. Stratum I - Steff to very steff, dark gray(sh brown FAT CLAY (CH) Stratum II - Very steff to hard, white CHALK Stratum II - Very steff to hard, white CHALK Stratum II - Very steff to hard, tan LEAN CLAY (CL) - with calcareous deposits from 6 to 10 feet - becomes tan and light gray in color below 8 feet SS 5	STRATED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION UNING CONTRACTOR BEA LING METHOD Dry Auger SED BY D. Rochs CHECKED BY R. Burge SED Groundwalter not encountered during drilling operations. MATERIAL DESCRIPTION Stratum II - Stiff to very stiff, dark gray/sish brown FAT CLAY (CH) Stratum II - Very stiff to hard, white CHALK Stratum III - Very stiff to hard, white CHALK Stratum III - Very stiff to hard, tan LEAN CLAY (CL) - with calcaneous deposits from 6 to 10 feet - becomes tan and light gray in color below 8 feet SS 6	STRATED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION GROUND WATER LEVELS: LING CONTRACTOR BEA AT TIME OF DRILLING — AT TIME OF DRILLING — AFTER DRILLING	STRATED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION UNING CONTRACTOR BEA LING METHOD Dry Auger SED BY D. Rochs CHECKED BY R. Burge SED Groundwater not encountered during drilling operations. MATERIAL DESCRIPTION Stratum II - Stiff to very stiff, dark gray/sh brown FAT CLAY (CH) Stratum III - Very stiff to hard, white CHALK Stratum III - Very stiff to hard, white CHALK Stratum III - Very stiff to hard, tan LEAN CLAY (CL) - with calcareous deposits from 6 to 10 feet - becomes tan and light gray in color below 8 feet SS 8-13-16 (29) SS 17-37-50/37	STARTED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION HOLE UNING CONTRACTOR BEA LING METHOD _DVA Juger LED BY _D. Rocha	STARTED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION HOLE SIZE LING CONTRACTOR BEA GROUND WATER LEVELS: LING METHOD	STARTED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION HOLE SIZE 5'	STRATED 8/13/25 COMPLETED 8/13/25 GROUND ELEVATION MOLE SIZE 5"	STARTED B13/25 COMPLETED B/13/25 GROUND ELEVATION HOLE SIZE 5"

Ana Jaime |

Foundation

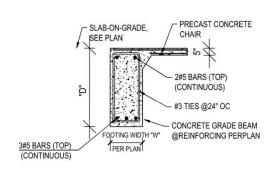


TYP. INTERIOR BEAM DETAIL

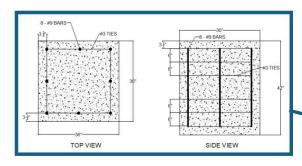


TYP. SLAB-ON-GRADE AT BEAM DETAIL

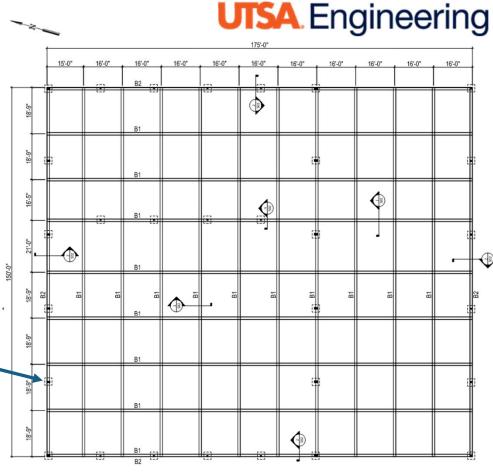
Interior beams 12 in × 24 in.
Each beam includes 4-#5 bars
(2 bottom + 2 top) continuous
Shear reinforcement uses #3 ties @ 24" oc



TYP. EXTERIOR BEAM DETAIL



TYP. PEDESTAL DETAIL



Exterior beams 12 in × 24 in Reinforced with 5-#5 bars (2 top + 3 bottom) continuous

Shear reinforcement consists of #3 ties @ 24" oc

Slab on grade

Slab thickness 5 in Reinforced with #4 @ 12" oc each direction

Ana Jaime |

Water Resources

Existing and Proposed Site Conditions

Per COSA Storm Water Design Criteria Manual, June 2025

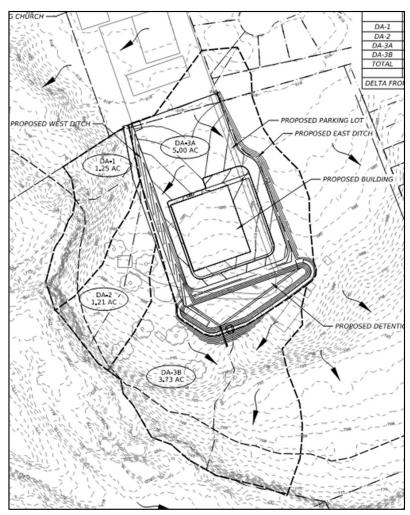
Analyzed 5 yr, 25 yr and 100 yr Storm Intensities

Existing SCS Unit Hydrograph Peak Flow Calculations

Subbasin ID	Area		Base CN	Impervious Cover %	Тс	Q1	Q5	Q25	Q100
	(AC)	(SQMI)			(MIN)	(CFS)	(CFS)	(CFS)	(CFS)
DA-1	1.4	0.00214	76	-	8.4	0.9	2.7	5.4	8.20
DA-2	1.2	0.00191	76	-	7.7	0.8	2.4	5.0	7.50
DA-3A	4.9	0.00759	76	5	11.9	3.6	9.2	17.9	26.50
DA-3B	3.7	0.00583	76	-	5.8	2.7	8.1	16.5	24.90
TOTAL	11.2					8.0	22.4	44.8	67.10

Proposed SCS Unit Hydrograph Peak Flow Calculations

Subbasin ID	Area		Base CN	Impervious Cover %	Тс	Q1	Q5	Q25	Q100
	(AC)	(SQMI)			(MIN)	(CFS)	(CFS)	(CFS)	(CFS)
DA-1	1.3	0.00195	76	-	8.4	0.8	2.4	4.9	7.50
DA-2	1.2	0.00189	76	-	7.7	0.8	2.4	4.9	7.40
DA-3A	5.0	0.00781	80	55	8.1	13.8	22.7	34.5	45.30
DA-3B	3.7	0.00583	76	-	5.8	2.6	7.8	15.9	24.10
TOTAL	11.2					18.0	35.3	60.2	84.30
DELTA FRO	M EXISTING					10.0	12.9	15.4	17.20



Brian Deringer

Water Resources

Proposed Detention Pond Plan

Goal: Detain excess runoff from additional impervious pavement.

Pond Top: 801.00'

Pond Bottom: 797.00'

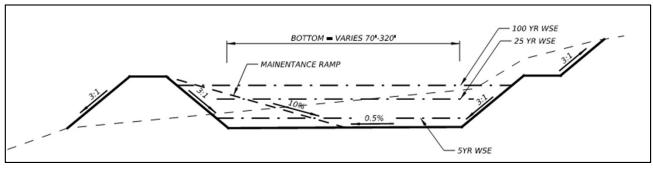
Emergency Weir: 800.00'

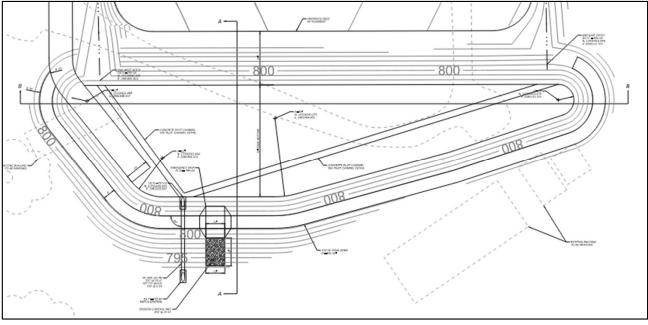
Freeboard: 0.90'

Outlet: 18" RCP

Receiving Ditches analyzed and sized using Rational Method and Flow Master. Consisting of a grass-lined section and 6:1 sides.









Water Resources

Proposed Detention Pond Hydrology and Hydraulics

Designed using HEC-HMS 4.12.

Hydrograph Shown is 100 yr storm at 6-hour duration

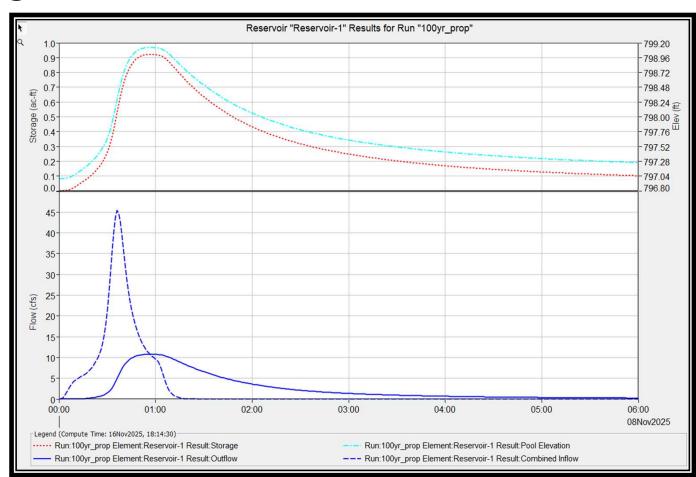
Peak Inflow of 45.3 cfs at 36 mins.

Peak Pond Water Surface Elevation 799.10' at 55 mins.

POND ELEVATION/STORAGE SUMMARY

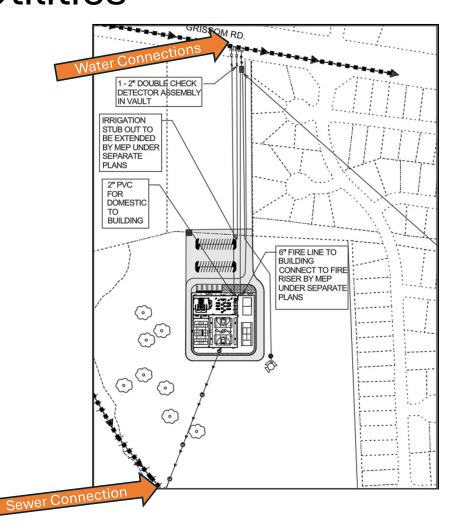
Elevation (FT)	Incremental Volume (CF)	Cumulative Volume (CF)	Acre Feet	Surface Area (SF)	Acre
797.00	0.77	0.77	0.00	16632.43	0.38
797.50	8576.69	8577.46	0.20	17679.55	0.41
798.00	9104.33	17681.79	0.41	18742.08	0.43
798.50	9639.00	27320.79	0.63	19818.56	0.45
799.00	10180.76	37501.55	0.86	20909.07	0.48
799.50	10729.57	48231.12	1.11	22013.78	0.51
800.00	11285.29	59516.41	1.37	23132.56	0.53
800.50	11848.45	71364.86	1.64	24265.58	0.56
801.00	12418.55	83783.41	1.92	25412.73	0.58

Storm	WSE (FT)	Peak Inflow (CFS)	Peak Discharge (CFS) 4.70		
5 YR	798.20	22.70			
25 YR	798.70	34.50	8.00		
100 YR	799.10	45.30	10.80		



Utilities





Utility Services & Sizing:

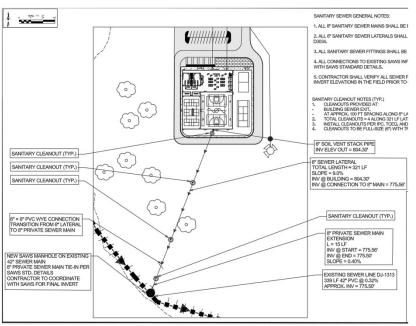
- 2" domestic water line (Q = 57gpm)
- 2" irrigation water line (Q = 60gpm)
- 6" fire line to fire riser room for sprinklers & onsite hydrant
- 6" sanitary sewer lateral (40 EDUs)
- Electrical service underground to the building

Irrigation design flow of 60 gpm is based on commercial rotor zone sizing, where 18-24 heads operating at 2-3gpm per head (Irrigation Association/Manufacturer Specs)

Domestic 2" line sized using IPC Table E103.3(2)&(3) (152 FU = 57gpm)

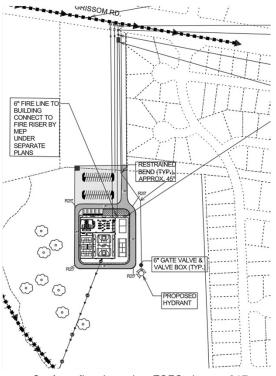
6" fire line required per IFC/NFPA for 1500 gpm fire flow for 2 hours

Utilities



Water & Sewer Main Connections:

- Connect to the **existing 8-in SAWS water main** on Grissom Road to serve the 2" domestic, 2" irrigation, and 6" fire line (≈670 ft to the building).
- Install **15 ft of new 8-in private sewer main** to tie the building's 6" lateral into the existing 42" SAWS sewer main.



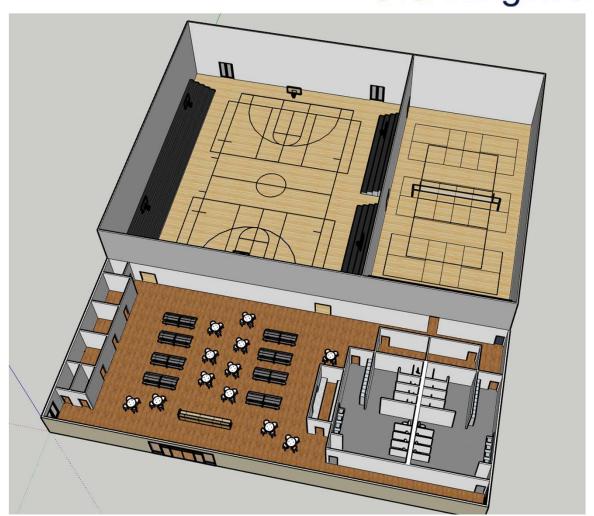
Sanitary flow based on TCEQ chapter 217: (15 gpd/person)
Occupancy based on IBC table 1004.5 (50 SF/person)

Structure

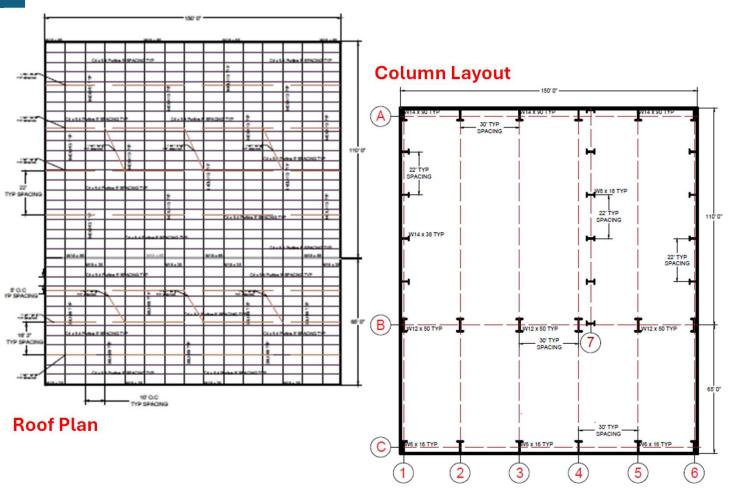
- Lobby/Entrance- 150' x 65' x 12'
- Gym- 150' x 110' x 35'
- 1:96 Pitched roof
- Steel Structure with some masonry exterior

Considerations

- IBC 2024, ASCE 7-22, AISC 360, SJI, TMS 402
- D = 16.3psf, L = 12psf, S = 14psf W = 23.3psf
- 1.2D + 1.6S (control) = 42.4psf
- Lateral Loading = 23.3psf
- Axial load to columns = 58.4k,
 102.4, 160.8k



UTSA Engineering



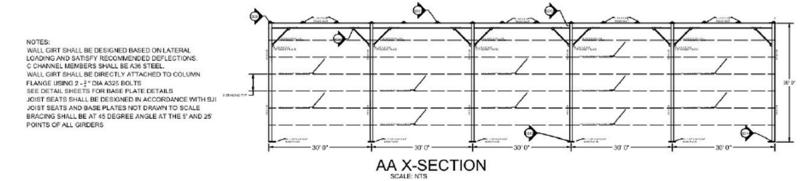
Members

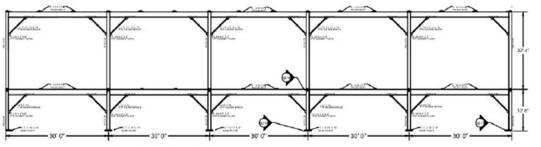
- Purlins
 - o C4 x 5.4, 5' O.C
- Trusses
 - o 64DLH13, 10' O.C
 - o 36LH09, 10' O.C
- Girders
 - o W18 x 65
 - o W18 x 35
- Columns
 - o W14 x 90, 30' spacing
 - \circ W12 x 50, 30' spacing
 - o W6 x 16, 30' spacing
 - o W14 x 38, 22' spacing
 - o W8 x 18, 22' spacing

UTSA Engineering

Bracing

- Truss
 - o L1½x1½x1/8
 - o 0.2S, 0.25S
 - o Diagonal every 4
- Girders
 - o L2 x 2 x 1/4
 - o L2 ½ x 2 ½ x ¼
 - o PL 3/8 x 6" x 8"
 - o A325 ½", ¾" bolts
- Wall girts
 - o 5' O.C
 - o C8 x 11.5
 - o C6 x 10.6
 - o C4 x 5.4
 - o 2-34" A325 bolts

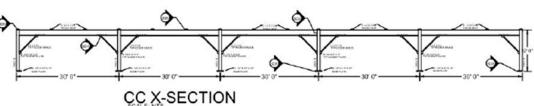




WALL GIRT SHALL BE DESIGNED BASED ON LATERAL LOADING AND SATISFY RECOMMENDED DEFLECTIONS. C CHANNEL MEMBERS SHALL BE A36 STEEL. WALL GIRT SHALL BE DIRECTLY ATTACHED TO COLUMN FLANGE USING 2-†* DIA A258 BOLTS. SEE DETAIL SHEETS FOR BASE PLATE DETAILS. JOIST SEATS SHALL BE DESIGNED IN ACCORDANCE WITH SJI JOIST SEATS AND BASE PLATES NOT DRAWN TO SCALE BRACING SHALL BE AT 45 DEGREE ANGLE AT THE 5' AND 25' POINTS OF ALL GROERS.

BB X-SECTION

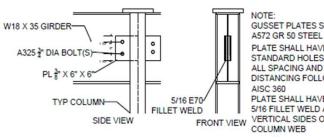
NOTES:
WALL GIRT SHALL BE DESIGNED BASED ON LATERAL
LOADING AND SATISFY RECOMMENDED DEFLECTIONS.
C CHANNEL MEMBERS SHALL BE A38 STEEL.
WALL GIRT SHALL BE DIRECTLY ATTACHED TO COLUMN
FLANGE USING 2.- 3" DIA A325 BOLTS
SEE DETAIL. SHEETS FOR BASE PLATE DETAILS
JOIST SEATS SHALL BE DESIGNED IN ACCORDANCE WITH SJI
JOIST SEATS AND BASE PLATES NOT DRAWN TO SCALE
BRACING SHALL BE AT 45 DEGREE ANGLE AT THE 5" AND 25"
POINTS OF ALL GIRDERS



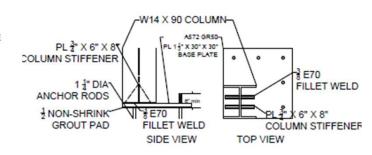
Connections

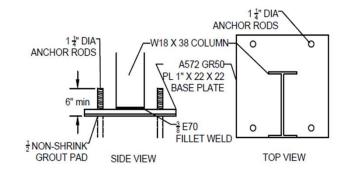
- Truss seat
 - Plate seat
 - o Angle Irons
 - o Fillet weld
- Girder Column
 - o (2) Gusset plate
 - Fillet weld
 - A325 Bolts
- Column Base Plate
 - Base plate
 - Anchor rods
 - Fillet weld
 - Stiffeners

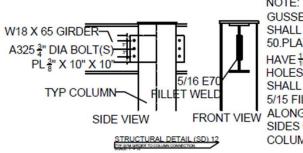
UTSA Engineering



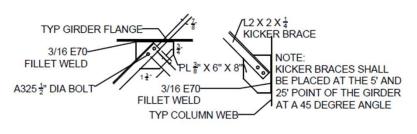
GUSSET PLATES SHALL BE PLATE SHALL HAVE 18 STANDARD HOLES WITH ALL SPACING AND DISTANCING FOLLOWING PLATE SHALL HAVE 6" OF 5/16 FILLET WELD ALONG VERTICAL SIDES ONTO

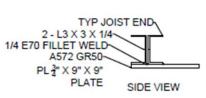






NOTE: GUSSET PLATES SHALL BE A572 GR **50.PLATE SHALL** HAVE 18 STANDARD HOLES. PLATE SHALL HAVE 10" 5/15 FILLET WELD ALONG VERTICAL SIDES ONTO **COLUMN WEB**

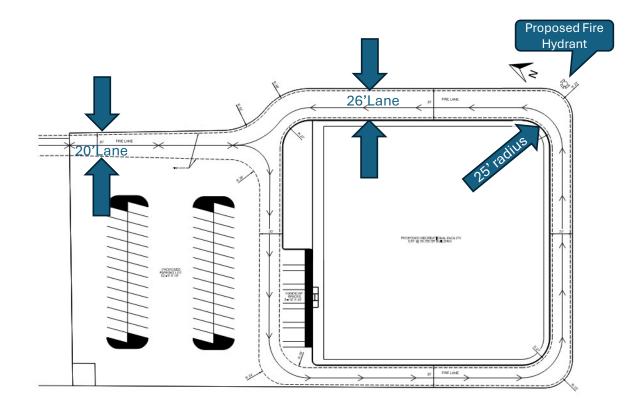




NOTE: PLATE SHALL REST ON COLUMN AS CAP FOR COLUMN TOP PROVIDE 1/4 FILLET WELD AROUND PERIMETER TO FASTEN PLATE TO MEMBER L BARS SHALL BE MIN 6" IN ACCORDANCE WITH

Fire protection plan

- Two existing fire hydrants located on Grissom Rd
- Proposing one fire hydrant
 - Southeast corner
- Building is fully sprinkled
- Meets minimum 20' lane width, 25' inner, & outer radii
 - 26' Fire Lane Access near proposed hydrant
- Per 2018 IFC 507.5.1, meets:
 - Hose pull by hand 200 LF
 - Hose pull by truck 550 LF





UTSA Engineering

- 52 Parking Spaces (2.0 per 1,000 sf)
 - 5 ADA-compliant
- Traffic Impact Analysis: 66 PHT < 76
 Trips
 - No TIA report required
- COSA Rough Proportionality Worksheet:
 - Utilizes existing driveway infrastructure
 - No off-site roadway or intersection improvements warranted

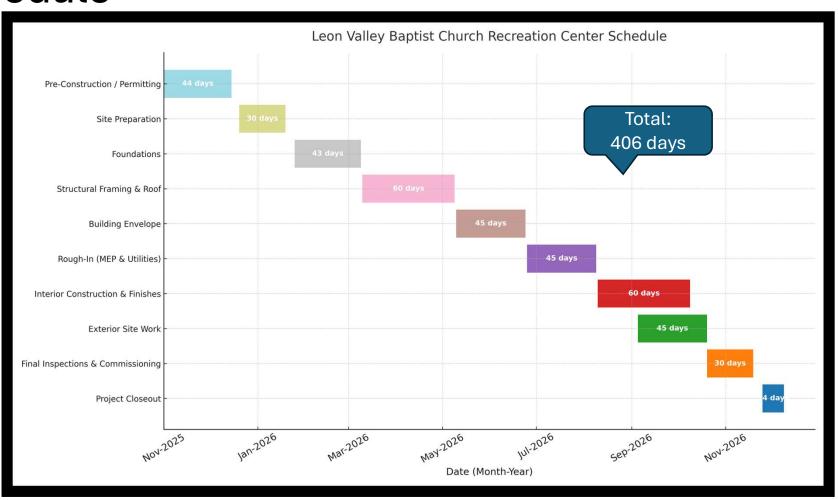
Use Classification	Minimum Vehicle Spaces	Maximum Vehicle Spaces				
Recreational Facility (ITE 495)	1.5 per 1,000 sf	10 per 1,000 sf				
Recreational Facility (26,250 sf)	40	262				

Traffic Impact Analysis (TIA) Threshold Worksheet										
Complete this form as an aid to determine if your project requires a Traffic Impact Analysis Study in accordance with UDC 35-502(b)(2). ITE 11th Edition.										
Project Name: Leon Valley Bap	tist Recreation:	al Center		Workshe	eet Prepared by:	Con	rad Montes			
Project Location: 7980 Grisse	m Road, San A	Antonio TX		Compan	y Lone Star En	zine	ering	On	vner 🔯 Owner's Agent	
Email: lonestarengineering@lor	estar.com			Address	1234 UTSA E	Boul	evard	Date:		
Jurisdiction: X COSA ICL		Other:					Zoning MDP	☐ Pla	t 🛛 Building Permit	
TIA Record Number (if applicable	e):			Associat	ed Record Number					
Proposed Type of Development:				itical Pea	k Hour: PM		Peak Hour Override:	PM		
Land Use	ITE Code	Project Size			Peak Hour Trip R	ate	Peak Hour Trips (PI	HT)	The rates and critical peak	
Recreational Community Center	495	26.25	1,000 SF GF	A	2.5		66		hour are automatically	
									calculated in this section	
									based on the linear rates of	
									ITE 11th edition. To change	
									the automatic peak hour	
Previous Development on Site:			Cr	itical Pea	k Hour:		Peak Hour Override:		calculator, check the Peak	
Land Use	ITE Code	Project Size	Unit		Peak Hour Trip R	ate	Peak Hour Trips (PI	HT)	Hour Override box and	
									input the correct peak hour. For custom or additional	
									fields, please use the second	
						\neg			page of the worksheet.	
						\neg		_	page of the worksheet.	
									1	
Total Trips: Please ensure land us	es for all lots/pa	rcels are includ	ed in the above	sections.				_		
Proposed Development Previous	is Developmen	t Differen	e in PHT	If	there is an increase	of 76	PHT and an increase o	f 10% of	the total PHT, a new TIA is	
56										

Conrad Montes |

UTSA Engineering

Schedule





Thank you!

Questions?

